



Licenciatura en Arquitectura

Nombre del alumno:

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Materia:

Estática para la arquitectura

Nombre del profesor:

Arq. Pedro Alberto García López

Cuatrimestre:

Tercero

Nombre de la actividad:

Unidad I: Estática (Ejercicios)

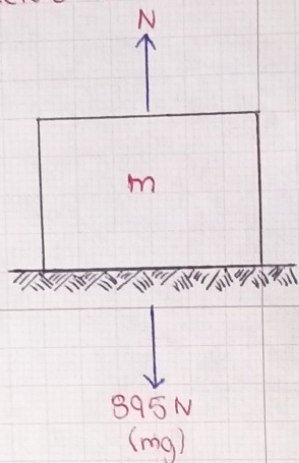
Fecha: 21 de mayo de 2023

"MOMENTOS Y CONDICIONES DE EQUILIBRIO"

21/05/2023

EJERCICIO 1:

①



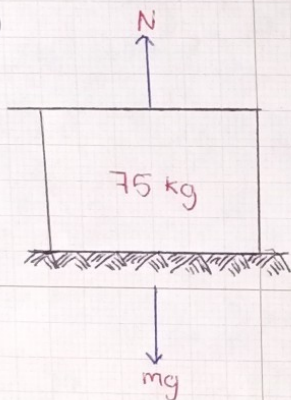
→ Encontrar la masa y comprobar el equilibrio

$$N \rightarrow \frac{895 \text{ N}}{9.81 \text{ m/s}^2} = \underline{91.23 \text{ kg}}$$

$$mg \rightarrow \frac{895 \text{ N}}{9.81 \text{ m/s}^2} = \underline{91.23 \text{ kg}}$$

$$\sum F_x = 0 \rightarrow 91.23 - 91.23 = \underline{0}$$

②



→ Comprobar equilibrio

$$N \rightarrow 75 \text{ kg} (9.81 \text{ m/s}^2) = \underline{735.75 \text{ N}}$$

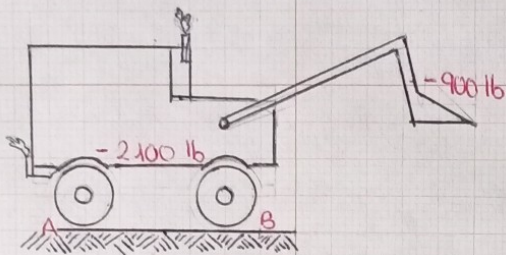
$$mg \rightarrow 75 \text{ kg} (9.81 \text{ m/s}^2) = \underline{735.75 \text{ N}}$$

$$\sum F_y = 0 \rightarrow 735.75 - 735.75 = \underline{0}$$

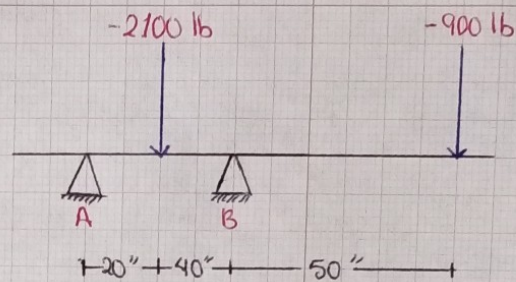
21/05/2023

EJERCICIO 2:

→ Un tractor de 2100 libras se utiliza para levantar 900 libras de grava, determine la reacción de cada uno de sus ejes de sus llantas.



① D.C.L



② EM

$$M_A \rightarrow [(-2100 \text{ lb})(20'')] + [(R_B)(60'')] + [(-900 \text{ lb})(110'')] = 0$$

$$-42,000 + R_B \cdot 60'' - 99,000 \text{ lb} \cdot \text{''} = 0$$

$$R_B = \frac{141,000 \text{ lb} \cdot \text{''}}{60''}$$

$$R_B = \underline{2,350 \text{ lb}}$$

③ $\Sigma F_y = 0$

$$R_A - 2100 \text{ lb} + 2350 \text{ lb} - 900 \text{ lb} = 0$$

$$R_A - 650 \text{ lb} = 0$$

$$R_A = \underline{+650 \text{ lb}}$$

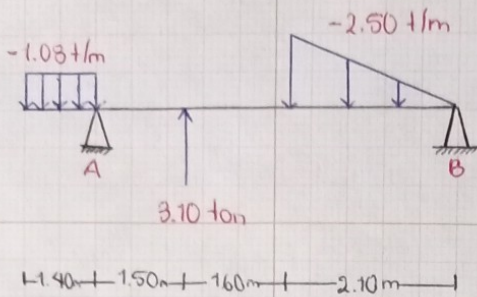
④ Comprobación

$$650 \text{ lb} - 2100 \text{ lb} + 2350 \text{ lb} - 900 \text{ lb} = 0$$

$$\underline{0 = 0}$$

21/05/2023

EJERCICIO 3:



① Cargas Puntuales

$$P_1 = W \cdot L$$

$$P_1 = (-1.08 \text{ t/m})(1.40 \text{ m})$$

$$P_1 = -1.512 \text{ ton}$$

$$UP_1 = L/2$$

$$UP_1 = 1.40 \text{ m} / 2$$

$$UP_1 = 0.70 \text{ m}$$

$$P_2 = \frac{W \cdot L}{2}$$

$$P_2 = \frac{(-2.50 \text{ t/m})(2.10 \text{ m})}{2}$$

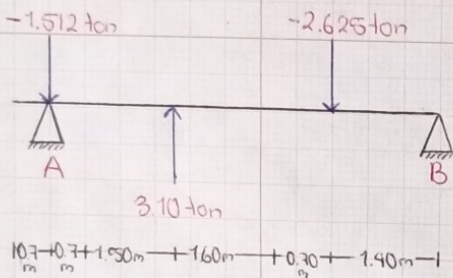
$$P_2 = -2.625 \text{ ton}$$

$$UP_2 = \frac{2}{3} L$$

$$UP_2 = \frac{2}{3} (2.10 \text{ m})$$

$$UP_2 = 1.40 \text{ m}$$

② D.C.L



③ $R_B \rightarrow \sum M_x = 0$

$$(1.512 \text{ ton} \cdot 0.70 \text{ m}) + (3.10 \text{ ton} \cdot 1.50 \text{ m}) + (-2.625 \text{ ton} \cdot 3.80 \text{ m}) + R_B \cdot 5.20 \text{ m} = 0$$

$$1.0584 \text{ ton} \cdot \text{m} + 4.65 \text{ ton} \cdot \text{m} - 9.975 \text{ ton} \cdot \text{m} + R_B \cdot 5.20 \text{ m} = 0$$

$$-4.2666 \text{ ton} \cdot \text{m} + R_B \cdot 5.20 \text{ m} = 0$$

$$R_B = \frac{4.2666 \text{ ton} \cdot \text{m}}{5.20 \text{ m}}$$

$$R_B = 0.82 \text{ ton}$$

④ $R_A \rightarrow \sum F_y = 0$

$$-1.512 \text{ ton} + R_A + 3.10 \text{ ton} - 2.625 \text{ ton} + 0.82 \text{ ton} = 0$$

$$-4.137 \text{ ton} + R_A + 3.92 = 0$$

$$R_A = 0.217 \text{ ton}$$

⑤ Comprobación

$$-1.512 \text{ ton} + 0.217 \text{ ton} + 3.10 \text{ ton} - 2.625 \text{ ton} + 0.82 \text{ ton} = 0$$

$$0 = 0$$