

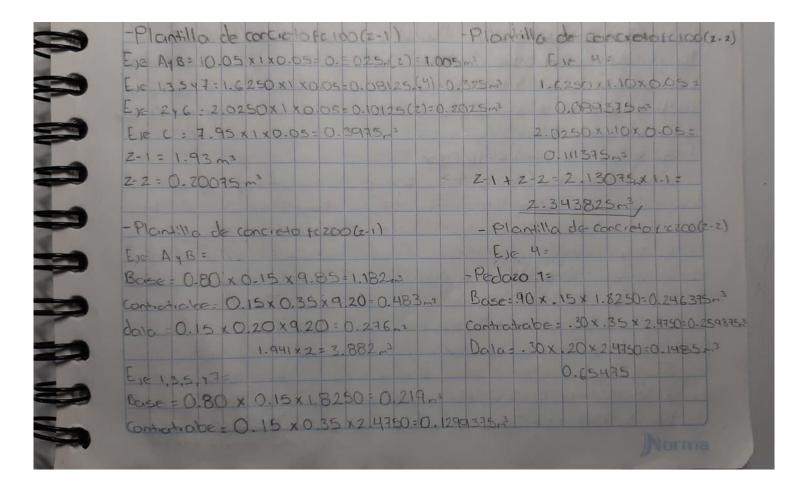
## CUANTIFICACIÓN

Taller de construcción

Carolina Del Rocío Ramírez Hernández ARQ. PEDRO ALBERTO GARCÍA LOPEZ

Talora de Datas	
	Resultados
	- 53. 56m²
Excavação	- 63.709425-
- XCONDEGO - 1 (2-3):	
Acero Nº3	- 39pz - 264.824kg
Acero NY	- 8pz - 95.577405kg
Acero NºS	
Acero Nº2	70 - 121 01024
Acero N2-	- 38pz - 111.6108kg
Zapara (0.11da (2.2)=	79 502.224
Acero Nº3	
Acero N5	- 2pz - 22.955994 kg
Accio Nº2	1
Da10 D-1	- 8pz - 42.95 m
Da10 D-2=	
Aceto Nº2	-1-
Acero Nº3	- 3pz - 14.0847968kg
Muro de enrase de cirrentación.	- 271pz
Plantilla de concreto te 100 kg/m² -	- 2.343825 m3
Plantillo de concreto rezockylne.	- 10.0043933563
Cashillo K-1	- 12pz - 72m
Acero Nº 2	
Acero N° 3	00.066.149
Cashillo K-3=	- 17pz - 111.93216kg
Acero N-2	20
Acero 10.4	20pz-57.9684kg
	- 9pz - 100-4976kg

	) Pre	3000	ore	5 =																	
			Circ																		
	- Lin	n Die	703																		
	Dasc:				. (	1.	10	X	5.8	30	2 5	3.3	360	2							
	orgo										1										
	-Exc	NOU	ion	(Z	-()									-E;	con	cic	10	2 (2	2-2	) =	
	e Ay					.15	= 11	55	75	3×	2 = 2	23.11	5~3	E	Je L	1=	1.6	25	Ox '	1-10	X1.15
	e 1,3,5																			25	
	= 2 y s														2.	02	50	x 1-	10 x	1.1	5=
	- C =										1 1				1	1	2.5	61	525	3	
	2-1= 2										Z-	1 +	Z- ;	2 = .	40	1.0	FO.	25	X	1.30	5=
2	7-2= 4	1.61	725~	3													6	3	. 70	194	25



	9
z-1 (Concretorezoo)	Z-2 (correcto (ctoo)
dala= 0.15 x 0.20 x 2.4750= 0.07425m3	- Petroc 25
0.4231875 x 4=1.69275 m3	Boses 90 x . 15 x 2.27 50:0.300315.2
Ese 246= 3	Contraticlos = .30 + .3- x 2.845 = 0-301835 3
Base = .80 x .15 x 2.2250 = 0.26 7m3	Dolo: .30x.20x7.875:0.1725.43
Contratratoe = .15 x . 35 x 2.875 = 0.1509375 m3	0.77475,3
Dala= .15 x .20 x 2.875=0.08625m3	
0.5041875x2=1.008375x3	1 2 z-1 = 8.098875 m3 3 4 4 1
Exe C =	92-2=1.42952
Base = .80 x .15x 7.75 = 0.93m3	
Contratable: .15 x . 35 x 7.10: 0.37275 m3	Z-1 + Z-2 = 9.52 8375 x1.05=
Dala: .15x,20 x 7.10: 0.213m3	10.00479875m3
1.51535 ~3	

Transversall	Ele	A		"long I dion"	05
long= .80+.24=1.04	In	13 14		long = 9.851.24 = 10.09.	
Pza= 1.625/.20 + 1= 9	.125=10			10.09m (4pz)=40.36m	
1-04mx 10pz = 10-4m x	2 = 20.8 m		-	Total en ese No 37.8m	
Pz = 1,25/,20+1 = 7.25=	8 - 14	1 1 1 1			
1.04 x 8p = 8.32 n	x 2 = 16.64~	3-17		The state of the s	-
Transversal		Eje B		Longitudinal=	
long = .80 + .24 = 1.04m	10			lang= 9.85 1.24 = 10.09.	
Pz = .25 1.20+1 = 2.25	= 3-2=1==			10.04 × 4 = 40.36	-
1.04m X 1pz = 1.04m(2)=	2.08-)			100000000000000000000000000000000000000	
Pz = 0.5751.20 +7= 3.87	5 = A - 2 = 2 pz	1 - 3		Total enere B = 59.08	
1-04m x 2pz = 2-08m x	2 = 4-16m	121-30	22/3		
Pz = 1.25/.20 +1 = 7.25 =	8-2-6pz		4	12/12/20 20/14	
1.04 x 6pz = 6.24m x 2	= 12.48my			Charles Charles	-
Transversal		ie C		Longit adinal:	
long = . 90 + . 24 = 1.04 m Pz =			=15	7.75+.24=7.99(4pz)=	
1.04 x 15 = 15.6 m x 2=	31.20			31.96m	-
				Total on eje c=63.16	

	Transversar Eje 1, 3.5 y 7	Nong to direct	
La	long = 0.80 + 0.24 = 1.04 =	lone 3 423 × 24 = 2 6 95 (4)	
	Pz = 1.8250/.20+7=10.125=11	14.66m x 4 = 58.64m	
3	1-04 × 11 = 1144 x 4 = 45-76-1	Total on one 13,5,7=104.40/	
	Transversal = Eje 2,6	Long Audina 1	7
3	10ng- 80 1.24=1-09m	long = 3.8250+.24=4.065×9,=	39.44
	P2= 2,22501.20 +1=12.125=13	16.24(2) = 32.564	A STREET
3	1.04 x 13 = 13.52 m x 2 = 27.04 m/	Total en eje 2 , 6 = 59:60/	
La			
3	Contratate Zapata 1 (Acero Nº3)=		
4	Ese A=	Eje 1, 3, 5, 7 =	
9	long= 9.20 + 0.24 = 9.44 mx 2 = 18.88 m/	long = 2.7754.24 = 3. DIS x Zpz =	
1	Eje 8:	6.03m x 4 = 24.12m/	
	long = 9.20 + 0.24 = 9.44 m x 2 = 18.88 m/	C de ar and a de as a 27	
	Eye C:	t je 2 1 6 =	at the same
	long= 7.10m + 0.24n= 7.34m × 2=14.68m/	long= 3.1750+0.24=3.415=x2=	123-12-16
8		6.83m × 2 = 13.66m/	

Totales	de Acero	N°3 en cod	Q E (2-1)=
E Je A = 96.	58m		E)c 1,3,5,7=128.52~
Exe B = 77.0			E E 16 2,6=73.26 m
Eje C = 77.8	4m	The Management of the Manageme	
		Tota	al dem en Acero Nº3 Zapata 1=
		Ede ejes	= 454.26 × 1.03=467.8878
		Pz	2 = 467.8878 = 12 = 38.99065
		P <sub>2</sub>	2 = 39
		K	9=0.566 × 4C7.8878
		K	9=264.824

Contratal	DE Acero Nº5	
Eje A = 9	185 + 0.40 = 10.2	25m x 2 pz = 20.5m
Ex B = 9.	85 +0.40 = 10.2	25nx 2pz=20.5m
		5 x 2 g z = 16.3 m
		3.825 x 2= 1.05 x 4 = 30.6m
		4.225 m x 2 = 2 = 8.45 m x 2 = 16.9 m
		Total de Aceronos do 2-1 en cimanta
		104.8m × 1.07 = 112.136
		Pz=112.136/12 = 9.34
		P2=10/

Contratrato	e Acero Nº 4 =	AN ALLENSE SERVICE
	.20 +.30= 9.50 x 2 = 19m	x2=38m
	10 + . 30 = 7 . 40 x2 = 14 . 8 m	
	7 = 2.775 + .3 = 3.075 × 2 =	
	3.175 1.30 = 3.475 x2 = 6.	
		Total de Acero Nº 41 de 2
	The same at the	91.3mx 1.05= 95.865m
		Pz = 95.865/12 =7.988
		Pz = 8/
		Kg=95.865 x 0.997 kg
		Kg=95.577405,

(contratable 05+, 1605 (A caro N°2)

L = . 15+. 15 +. 50 +. 50 = 1.3 +. 14 = 1.44 m

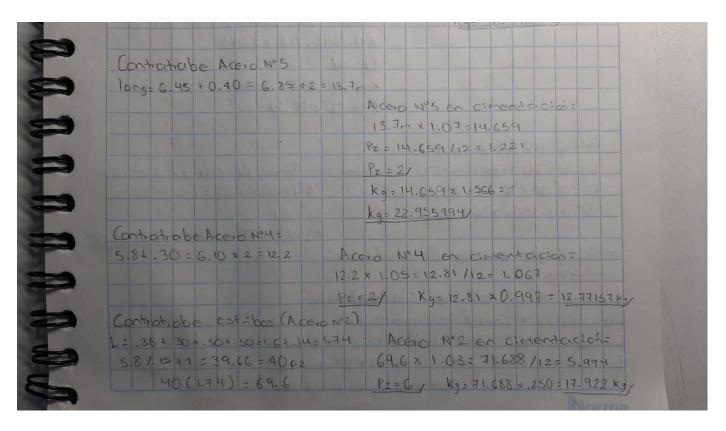
E pe A y B = 9.20 1.15 + 7 = 62.3 = 63 pz (1.44 m) = 90.72 x 2 = 181.44 m

E pe C = 7-10 1.15 + 1 = 48.3 = 49 pz (1.44) = 70.56

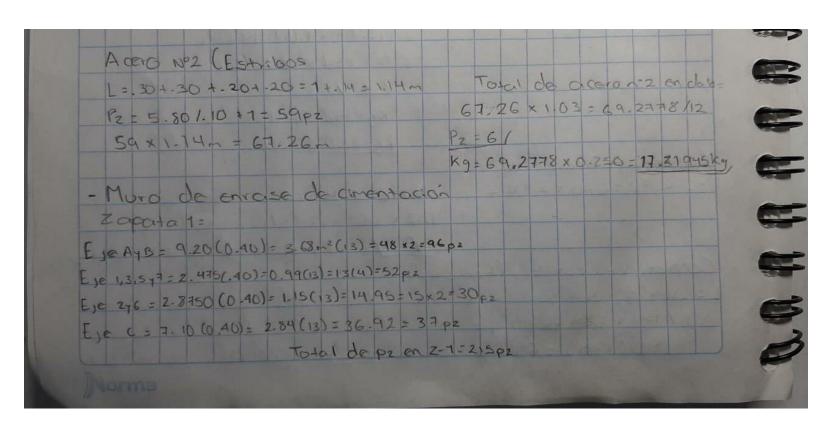
Total de acero N°2 = 6.6 1.3,5 y = 2.775 1.5 + 7 = 19.5 = 20 pz (1.44) = 28.8 x 4 = 115.2

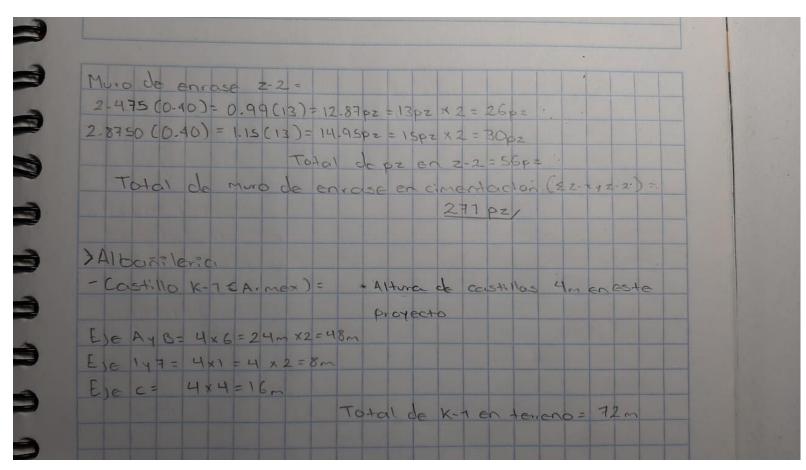
L = 2 y C = 3, 1751.15 + 7 = 22.16 = 23 (1.44) = 35.12 x 2 = 66.24 m Pz = 381 kg = 116.6108

- Zapata Corrido	z-2	Ace	10	2, 3	,,4	yd	5)	1				
Parilla Acero Nº 3		AN E									14	
Transversal (sacc	ion 1)		100		1		Trai	SIE	1021	VCS	200	21
0-90+0-24=1-14~	10000						P= = 2.	225	1-20	17=1	2-125	2 13
Pz = 1-8251-20+7=10	1.125=1	125		7.4			13 (1-1	40).	=14 -	82~	1	
11 (1.14m) = 12.54m			51	100	P	1						
Longitudinal:			1									
6.45+0.24 = 6.69.	(4)= 2	26-76	7		13	4	Ace	70	Nº3	en c	inendo	Dictor.
							· (z-	2)=	66	-2;	× 1.0	3=
Contratrabe							6	8-12	36 m	4-1		
Long = 5.8 + 0.24 = 6	.04 m x :	2 = 12	-08	~/			Pz	= 68	.186	/12	=5.6	8
							Pz:	6/				
							K	= 68	8-186	×C	5.566	
							ka	= 38	3. 59	3276	/	



-Dala 0-1(m)		
Ex Ay B = 9.20 = 184 E	6 1, 3, 5, 47 = 2, 775 = 11-12	
Epe c = 7.10m E)	2 y 6 = 3-175m = 6.35m	
T	tal dem de Dala 1=	
	42.95~/6~	
P	= 7.15	
	2=8/	
-Dala D-2 (Acero Nº27		
Acero nº3		
long = 5-8+0-24= 6-04		
6.04 × 402 = 24.		
	Total de acoro re3 en dala (0-2	)=
	24.8848m/12 = 2.073	
	Pz=3/	
	Kg=241,8848 × 0.566=	





- Kz ( Agao 3) =	Cantillas en
L= 4m Pz=6	6 x 4=24m (8) = 192 (103) = 197.76/12
	P2=16.48 = 17p=1
	kg=11193216xg/
[stribos Nº20	x-2)
	Pz = 4n1. 5 = 26.66 = 2711 = 2882
L=0.60+.14=0.7	4(2)=1.48m
	28(1.48~)=41.44~(8)=331-52
	331.52 (1-03) = 341.4656
	Pz = 341-4656/12=28-45
	Pz = 2902/
	Kg= 341.4650 x 0.250 =
	kg= 85.3664/

K-3 CA	CE10 104)=	
L=4~	P2 = 8	4x8=32m(3)=96x1.05=100.8
		PZ=100.8/1Z = 8.4
		Pz = 9
		Kg= 100.8 x 0.997 kg
		K2=100.4976
Estribo	5 Nº2 (K-3)	
		Pz=4m1.15=26.66=27+1=28pz
L=1-2+	.14 = 1-34(2)=2.0	68 28(2.68) = 75.04m(3) = 225-12
		225.12 (1.03): 231.8736
		Pz = 231-073 c/12 = 19-32
		Pz = 20/
		ky=231.8736×0.250=
		kg = 57.9684