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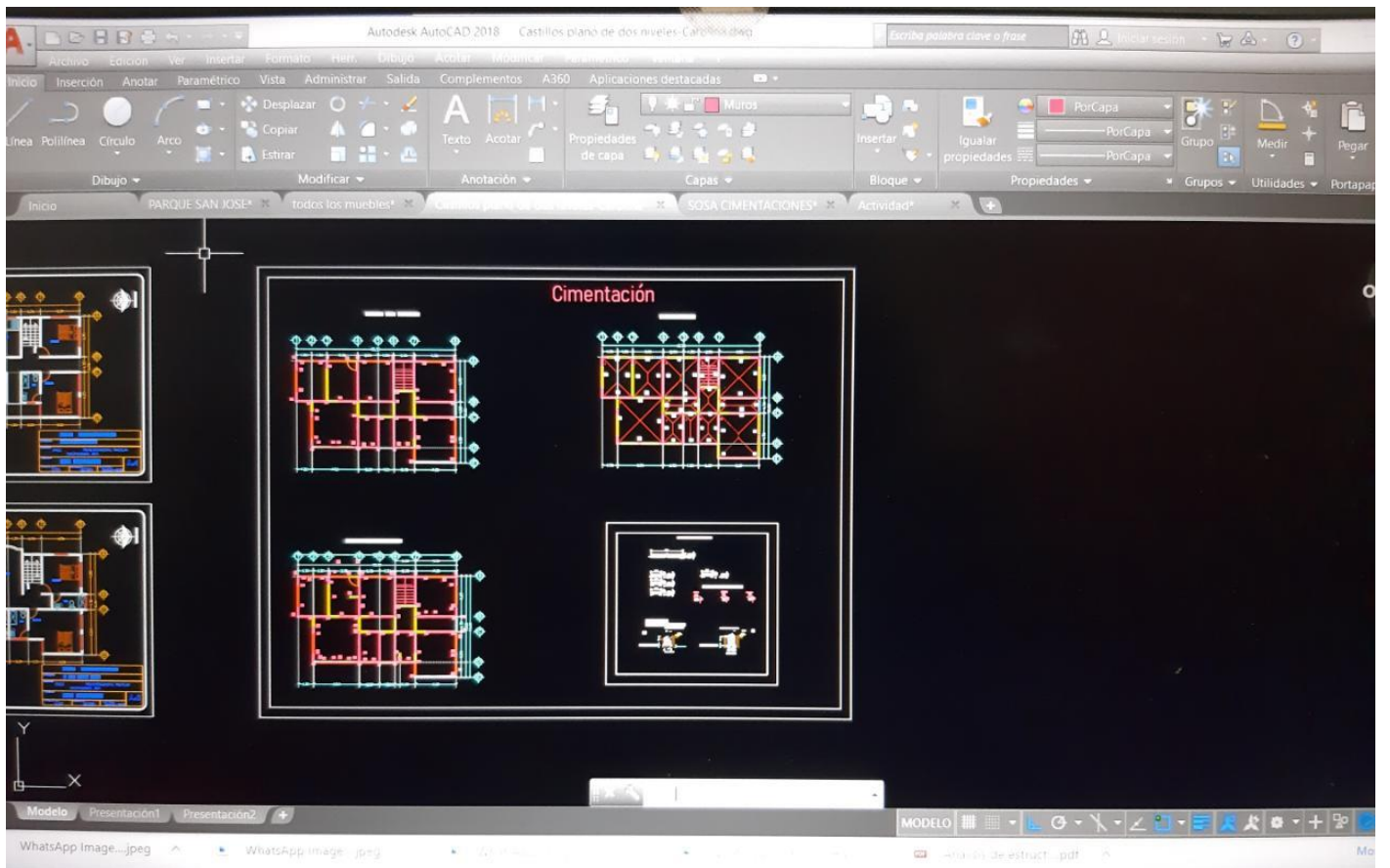
**Nombre del profesor: PEDRO ALBERTO
GARCIA**

**Nombre del trabajo: Zapata de
piedra dimensiones**

**Materia: Análisis de procesos
constructivos**

Grado: 5° cuatrimestre

Grupo: Arquitectura



Zapata de Piedra blanda

- > Eje 1 = (Tablero I) $E_{FCA} = 3.3758847134$ > Eje 8 = (Tablero XIII)
- Losas = $3.6375 (.5804/m^2) = 2.109951$ > $4.328625 / 3.9250$ Losas = $4.82 (.580) = 2.813$ > $5.9715 / 4.1250$
 $3.6375 (.6104/m^2) = 2.2188751$ > 1.1028342049 Losas = $4.85 (.610) = 2.9585$ > 1.304292858
- Muros = $2.5(0.271/m)(2) = 1.351/m$ $\Sigma = 2.5968343949$ Muros = $2.5(0.277/m)(2) = 1.351/m$
 $\times 1.3 =$
- Cerramientos = $.15 \times 20 \times 2.4 = .072(2) = .144$ 3.3758847134 Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .144(2) = .288$
- > Eje 2 = (Tablero VI) $\Sigma = 2.79829378531/m \times 1.3 = 3.6377819209$
- Losas = $5.0625 (.5804/m^2) = 2.936251/m$ > $6.024375 / 4.5$ > Eje A = (Tablero V)
 $5.0625 (.6704/m^2) = 3.0881251/m$ > 1.33875 Losas = $3.9986 (.580) = 2.31918$ > $4.158334/4$
 $3.9986 (.610) = 2.439146$ > 1.1895835
- Muros = $2.5(0.274/m)(2) = 1.35$ $\Sigma = 2.832751/m \times 1.3 =$ Muros = $2.5(0.277/m)(2) = 1.351/m$
 $\times 1.3 =$
- Cerramientos = $.15 \times 20(2.4)(2) = .144(2) = .288$ $3.6825751/m$ Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .144(2) = .288$
- > Eje 3 = (Tablero II) $\Sigma = 2.6835835 \times 1.3 = 3.48865855$
- Losas = $3.6375 (.5804/m^2) = 2.109951$ > $4.328625 / 3.9250$ > Eje B = (Tablero VI)
 $3.6375 (.6104/m^2) = 2.2188751$ > 1.1028343949 Losas = $5.0625 (.580) = 2.93625$ > $6.024375 / 4.5$
 $5.0625 (.610) = 3.088125$ > 1.33875
- Muros = $2.5(0.271/m)(2) = 1.351/m$ $\Sigma = 2.5968343949$ > $\times 1.3 =$ Muros = $2.5(0.277/m)(2) = 1.351/m$
 $\times 1.3 =$
- Cerramientos = $.15 \times 20(2.4)(2) = .144(2) = .288$ 3.3758847134 Losas = $5.0625 (.610) = 3.088125$ > 1.33875
- > Eje 4 = (Tablero VI) $\Sigma = 2.832751/m \times 1.3 = 3.6825751/m$
- Losas = $5.0625 (.5804/m^2) = 2.93625$ > $6.024375 / 4.5$ Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .144(2) = .288$
 $5.0625 (.6104/m^2) = 3.088125$ > 1.33875 Muros = $2.5(0.277/m)(2) = 1.351/m$
 $\times 1.3 =$
- Muros = $2.5(0.274/m)(2) = 1.351/m$ $\Sigma = 2.832751/m \times 1.3 =$ $\Sigma = 2.832751/m \times 1.3 = 3.6825751/m$
 $\times 1.3 =$
- Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .144(2) = .288$ $3.6825751/m$ > Eje C = (Tablero XIII)
 $4(.580) = 2.32$ > $4.76/4 = 1.19$

Muros = $2.5(0.271/m)(2) = 1.351/m$ $\Sigma = 2.5968343949$ $\times 1.3 =$ Eje B = (Tablero VI)
 Cerramientos = $.15 \times 20(2.41)(2) = .1441/m$ 3.3958847126 Losas = $5.0625(.580) = 2.93625$ $\times 1.3 = 6.024375/m$
 $5.0625(.610) = 3.078125$ 1.33875

> Eje 4: (Tablero VI)
 Losas = $5.0625(.580) = 2.93625$ $\times 1.3 = 6.024375/m$
 $5.0625(.610) = 3.078125$ 1.33875 Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .1441/m$

Muros = $2.5(0.271/m)(2) = 1.351/m$ $\Sigma = 2.832751/m \times 1.3 = \Sigma = 3.6825951/m$
 Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .1441/m$ $3.6825951/m$ Eje C = (Tablero XIII)

> Eje 5 = (Tablero XI)
 Losas = $2.025(.580) = 1.1745$ $\times 1.3 = 2.40975/m$
 $2.025(.610) = 1.23525$ $0.78565853661/m$ Muros = $2.5(0.271/m)(2) = 1.351/m$
 Losas = $4(.580) = 2.32$ $\times 1.3 = 4.7614$
 $4(.610) = 2.44$ 1.19

Muros = $2.5(0.271/m)(2) = 1.351/m$ $\Sigma = 2.2776585366$ Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .1441/m$
 Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .1441/m$ 2.9609560976 $\Sigma = 2.6847/m \times 1.3 = 3.48921/m$

> Eje 6 = (Tablero III) $\times 1.3 =$ Eje D = (Tablero VI)
 Losas = $3.8062(.580) = 2.207616$ $\times 1.3 = 4.5297813920$
 $3.8062(.610) = 2.321782$ 1.1539816561 Losas = $5.0625(.580) = 2.93625$ $\times 1.3 = 6.024375/m$
 Losas = $5.0625(.610) = 3.078125$ 1.33875

Muros = $2.5(0.271/m)(2) = 1.351/m$ $\Sigma = 2.6479816561 \times 1.3 =$ Muros = $2.5(0.271/m)(2) = 1.351/m$
 Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .1441/m$ 3.4423761579 Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .1441/m$
 $\Sigma = 2.832751/m \times 1.3 = 3.6825951/m$

> Eje 7 = (Tablero XIII) $\times 1.3 =$ Eje E = (Tablero XIII)
 Losas = $4.85(.580) = 2.813$ $\times 1.3 = 5.7715/m$
 $4.85(.610) = 2.9585$ $1.30429375 = 1.1/1$ Losas = $4(.580) = 2.32$ $\times 1.3 = 4.7614$
 Losas = $4(.610) = 2.44$ 1.19

Muros = $2.5(0.271/m)(2) = 1.351/m$ $\Sigma = 2.79829378531/m$ Muros = $2.5(0.271/m)(2) = 1.351/m$
 Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .1441/m$ 3.6377819209 Cerramientos = $.15 \times 20 \times 2.4 \times 2 = .1441/m$
 $\Sigma = 2.6847/m \times 1.3 = 3.48921/m$

Sobrecarga máxima de cimentación en zapata intermedia

$$\varepsilon_{FCQ} = 3.682575 \text{ t/m}$$

Datos =

$$\varepsilon_{FCQ} = 3.682575 \text{ t/m}$$

$$\text{Corona min} = 30 \text{ cm} = 0.30 \text{ m}$$

$$r = 8 \text{ ton/m}^2$$

$$L = 100 \text{ cm} = 1 \text{ m}$$

Formula

$$\frac{\varepsilon_{FCQ}}{A} \cdot r = \frac{\varepsilon_{FCQ}}{r} = A$$

$$B = \frac{A}{L} \quad B \geq 60 \text{ cm}$$

$$T = \frac{B - c}{2}$$

$$H = \text{tang}(60^\circ) \cdot T \text{ (verto)}$$

Sustitución =

$$\frac{3.682575 \text{ t/m}}{8 \text{ ton/m}^2} = 0.46$$

$$A = 0.46 \text{ m}$$

$$B = \frac{0.46 \text{ m}}{1 \text{ m}} = 0.46 \text{ m} = 46$$

$$B = 0.60 \text{ m}$$

$$T = \frac{60 - 30}{2} = 15$$

$$T = 0.15 \text{ m}$$

$$H = \text{tang}(60^\circ) \cdot (15) = 25$$

$$H = 0.60 \text{ m}$$

Medidas mínimas de zapata intermedia:

$$0.60 \times 0.60 \times 0.30$$

$$T_{\text{alud}} = 0.15 \text{ m}$$

Sobrecarga máxima en cimentación de zapata de cilindro
3.6377819209 t/m

Datos

$$\Sigma FCG = 3.6377819209 \text{ t/m}$$

$$\text{Corona min} = 0.30 \text{ m} = 30 \text{ cm}$$

$$r = 8 \text{ ton/m}^2$$

$$L = 100 \text{ cm} = 1 \text{ m}$$

Formula

$$\frac{\Sigma FCG}{A} < r = \frac{\Sigma FCG}{L} = A$$

$$B = \frac{A}{L} \quad B \geq 0.60 \text{ m}$$

$$T = \frac{B - C}{2}$$

Sustitución:

$$\frac{3.6377819209 \text{ t/m}}{8} =$$

$$A = 0.4547$$

$$B = \frac{0.4547}{1 \text{ m}} = 0.4547$$

$$B = 0.60 \text{ m}$$

$$T = \frac{0.60 - 0.30}{2} = 0.15$$

$$T = 0.15 \text{ m}$$

Medidas mínimas de
zapata de cilindro

$$0.60 \text{ m} \times 0.60 \text{ m} \times 0.30 \text{ m}$$

$$\text{Talud} = 0.15 \text{ m}$$

$$H = \text{tang}(60^\circ) (15)$$

$$H = \text{tang}(60^\circ) (15) = 25$$

$$H = 0.60 \text{ m}$$